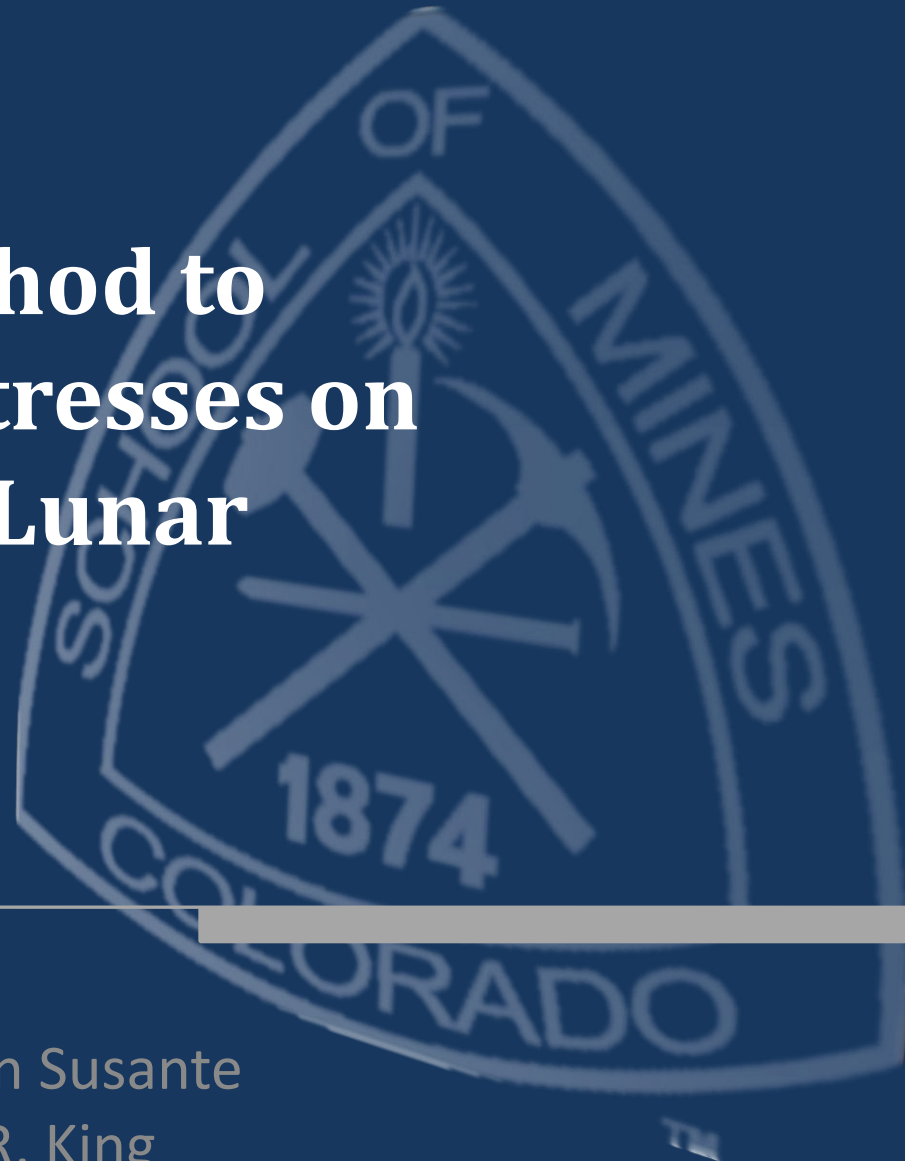


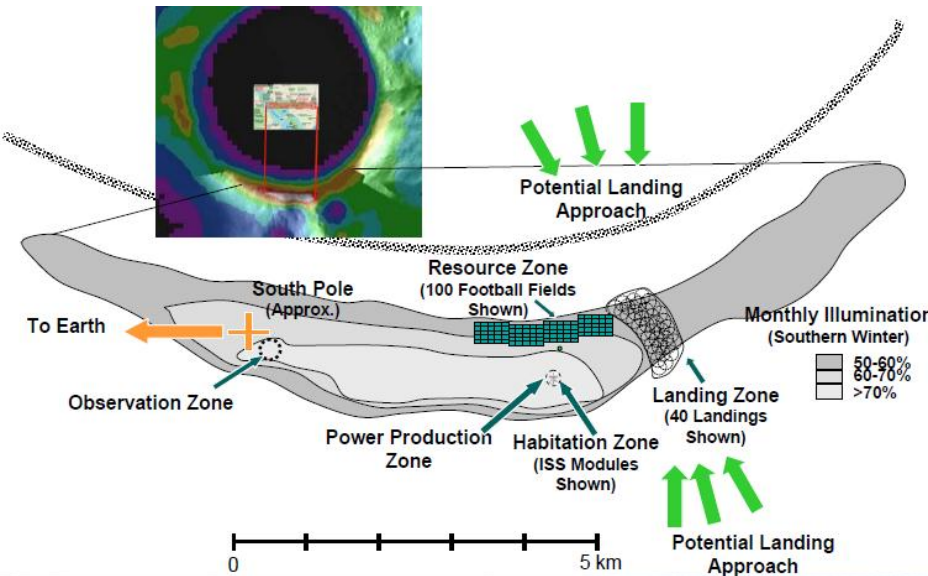
Finite Element Method to Calculate Forces and Stresses on Blades Excavating Lunar Simulants



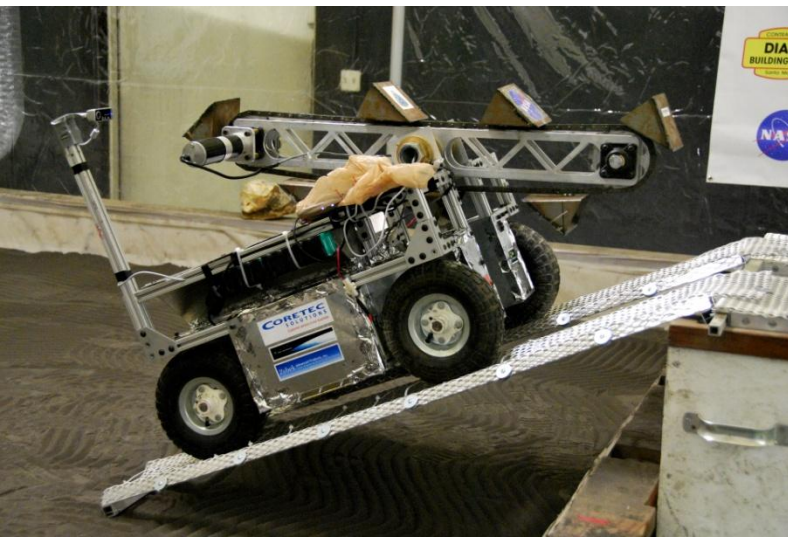
Thursday 6/10/2010
Joint Meeting of
Space Resources Roundtable
&
Planetary and Terrestrial
Mining Sciences Symposium

Paul van Susante
Dr. R. King

Why Lunar Excavation?



- NASA develops an outpost on a planetary surface
- requires handling of dry granular lunar sand and dust called regolith.
- high cost of bringing each kilogram to the lunar surface,
- machines must be small and lightweight.
- Current models not sufficient
 - All over the map
 - Designed for very specific applications
 - FEA models exist for cutting – rarely compared to exp

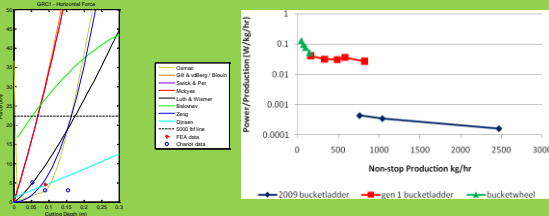


Lunar Excavation Goals

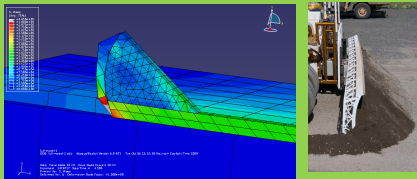
- Determining the input parameters of importance for analytical lunar soil-blade excavation models
- Determining the range of values for the important input parameters
- Experimentally determining the appropriate input parameter values for the constitutive model of JSC-1A lunar simulant for use in finite element analysis.
- Develop a finite element model to calculate the forces and stresses on a blade used to excavate lunar simulant and compare to the available experimental data

Overall CSM Integrated Picture

Analysis, theory



FE / analytical work



Prototypes



Lab - testing



Fidelity



Field testing

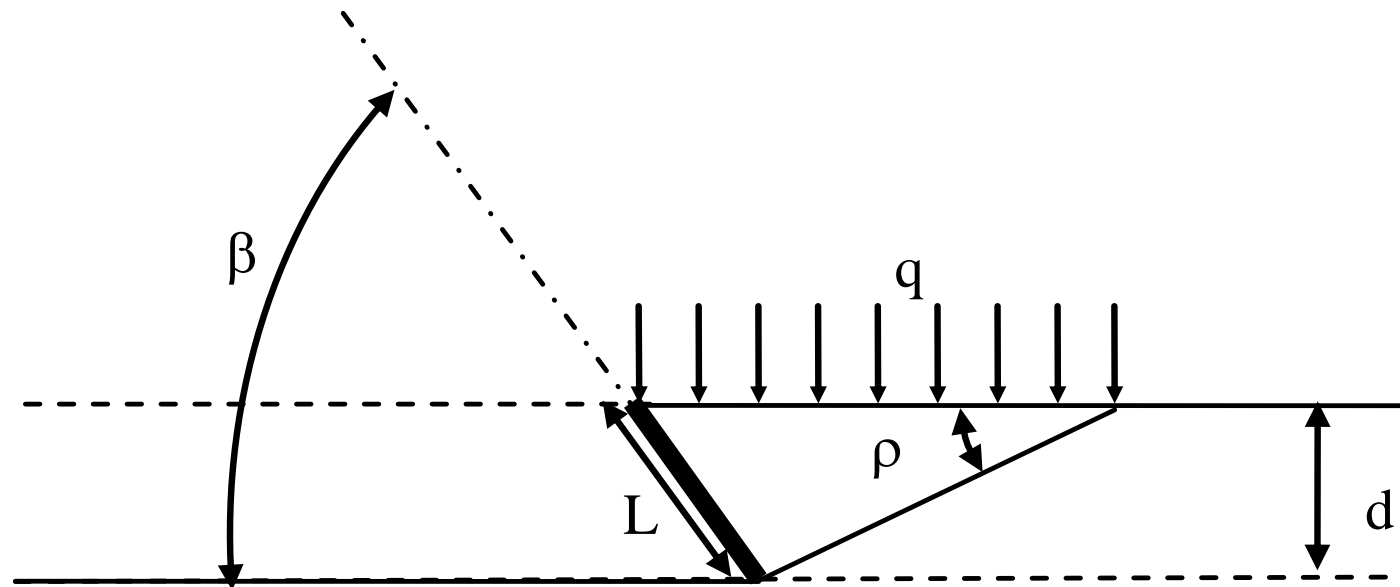


Science & System Engineering Inclusion

Simulations

- Determine input properties for soil models
 - Range, sensitivity
- Analytical models
- Finite Element Analysis

From Analytical Models



Geometry and Soil Mechanics

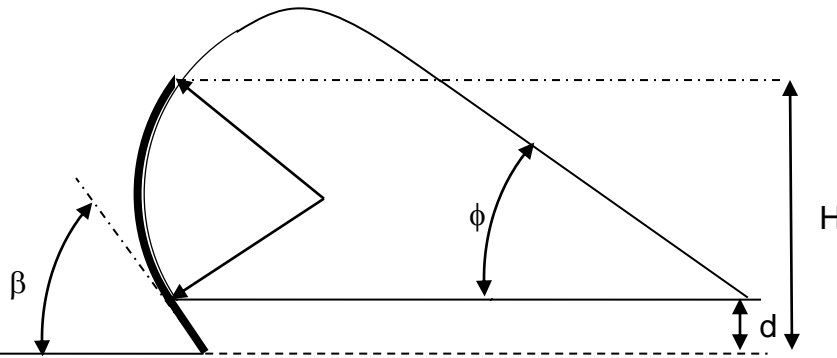
Input parameters of importance

$$T_{mckyes} = \left(\frac{wd}{\cos(\beta + \delta) + (\sin(\beta + \delta) \cot(\rho + \phi))} \right)$$

$$\left[\left(\frac{\gamma g d (\cot(\beta) + \cot(\rho))}{2} \right) + g q (\cot(\beta) + \cot(\rho)) + c(1 + \cot(\rho) \cot(\rho + \phi)) + C_a(1 - \cot(\beta) \cot(\rho + \phi)) + \left(\frac{\gamma^2 (\tan(\rho) + \cot(\rho + \phi))}{1 + \tan(\rho) \cot(\beta)} \right) \right]$$

$$H_{mckyes} = T_{mckyes} \sin(\beta + \delta)$$

$$V_{mckyes} = T_{mckyes} \cos(\beta + \delta)$$



Int friction angle

Ext friction angle

Unit Weight

Cohesion

Adhesion

 ϕ (deg) δ (deg) γ (g/cm³)c (kN/m²)c_a (kN/m²)

Min

Exp

Max

Min

Exp

Max

Min

Exp

Max

Min

Exp

Max

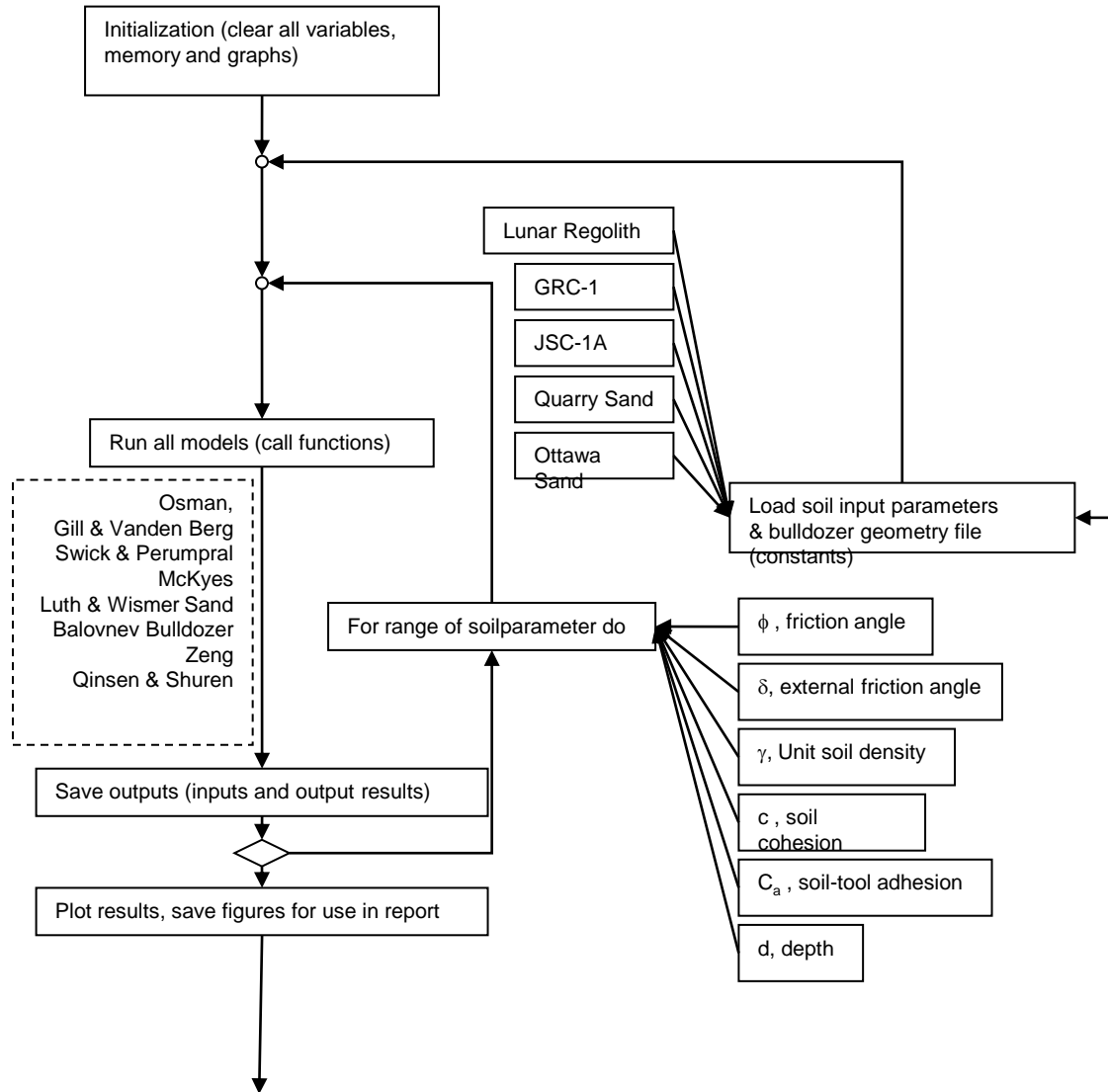
Min

Exp

Max

	Min	Exp	Max	Min	Exp	Max	Min	Exp	Max	Min	Exp	Max	Min	Exp	Max
Ottawa	29.1	35	41.6	11	15	32	0.9	1	1.07	0	1.51	3.03	0	0	0
Quarry	30	35	45	16	40	55	1.5	1.69	1.9	0	3.03	6.1	0	1	1
JSC-1A	37	50	57	16	40	64	1.45	1.5	1.9	0.827	3.8	6.83	0	1	1
GRC-1	30	40	45	16	40	64	1.58	1.73	1.89	0	4.96	9.92	0	1	1
Regolith	30	40	50	16	40	64	1.3	1.61	1.92	0.1	0.55	1	0	1	1

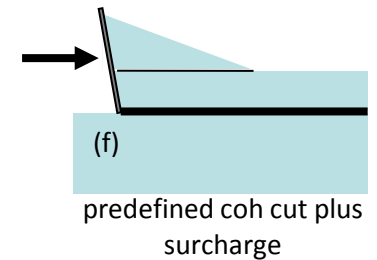
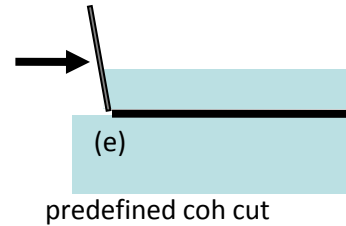
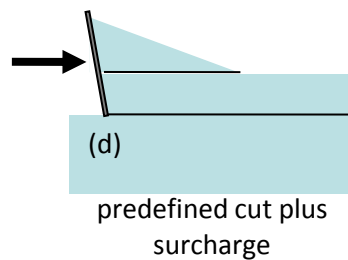
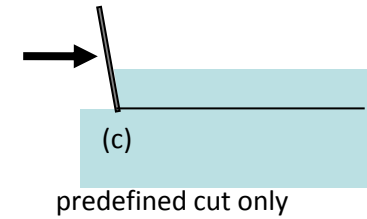
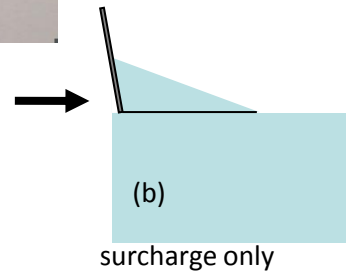
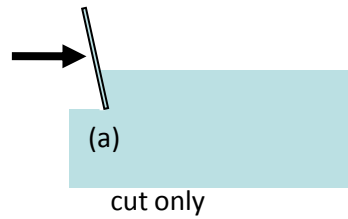
Limit Equilibrium Models

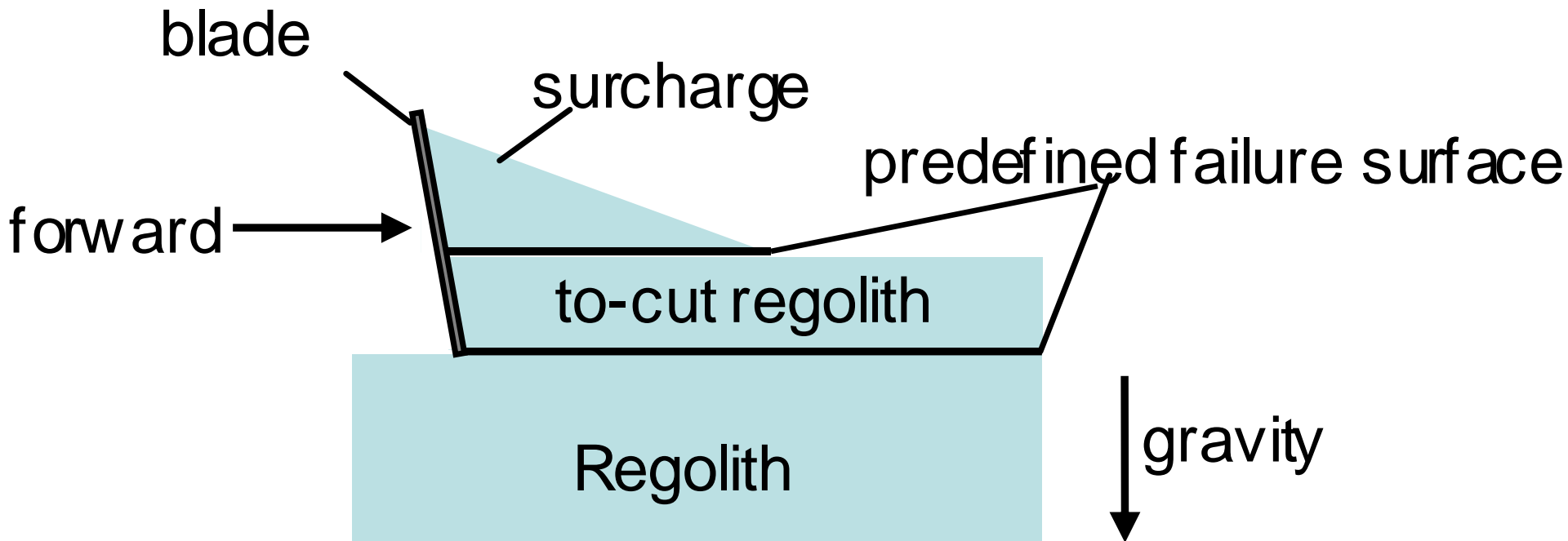


Force vs. depth results

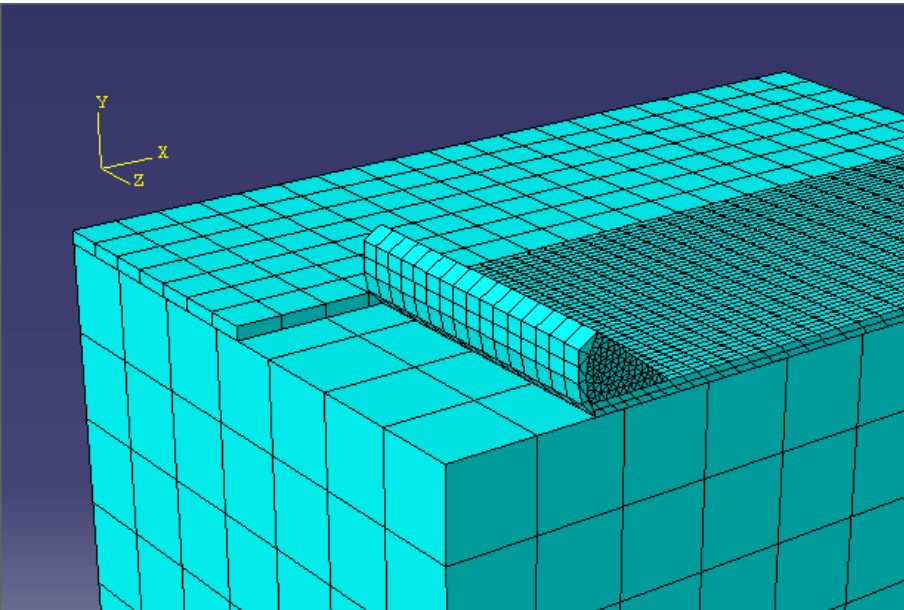
Horizontal Force (kN) for 30 cm	Osman	Gill & VdB / Blouin	Swick & P	McKyes	L&W sand	Balovnev	Zeng	Qinsen & Shuren
GRC-1	87.29	142.2	127.1	127.1	44.59	43.44	91.77	12.49
JSC-1A	<0	173.2	1.48	5.09	38.66	34.36	126.89	9.97
Regolith	<0	108.56	77.2	77.2	41.49	7.62	66.92	2.24
sand	29.24	34.97	19.23	19.23	25.77	10.57	11.33	3.749
Quarry sand	47.48	106.8	70.1	70.1	43.04	27.19	37.91	7.684

Max Depth (cm)	Osman	Gill & VdB / Blouin	Swick & P	McKyes	L&W sand	Balovnev	Zeng	Qinsen & Shuren
GRC-1	16.2	6.8	6.8	6.8	17.5	5	16.2	>>30
JSC-1A	n/a	5.8	n/a	n/a	19.5	3.5 or 41.8	14.5	64.5
Regolith	n/a	9.3	12	12	18.5	n/a	19.2	n/a
sand	25.4	21.7	35	35	22.5	>>30	38	>>30
Quarry sand	20.5	9	12	12	18	15.5	24	>>30





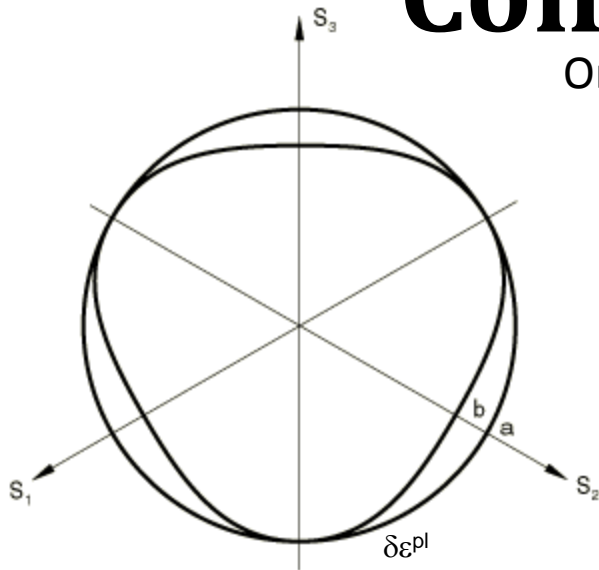
Finite Element Model



- Why FE
 - 3D
 - Forces & distributed stresses
 - More complex material behavior

Constitutive Model & Parameters

Original and Extended Drucker-Prager model



$$t = \frac{1}{2} q \left[1 + \frac{1}{K} - \left(1 - \frac{1}{K} \right) \left(\frac{r}{q} \right)^{\frac{1}{\psi}} \right]$$

Curve	K
a	1.0
b	0.8

 $\gamma = 1800 \text{ kg/m}^3$

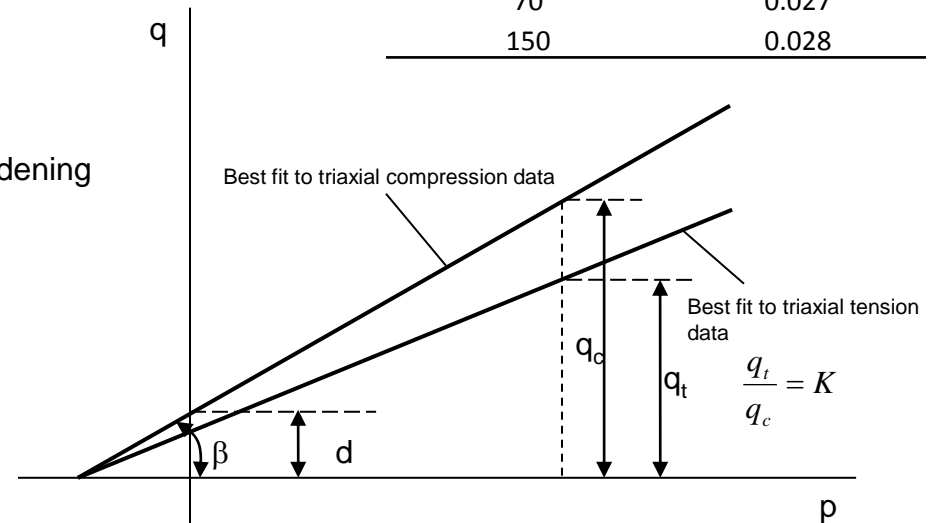
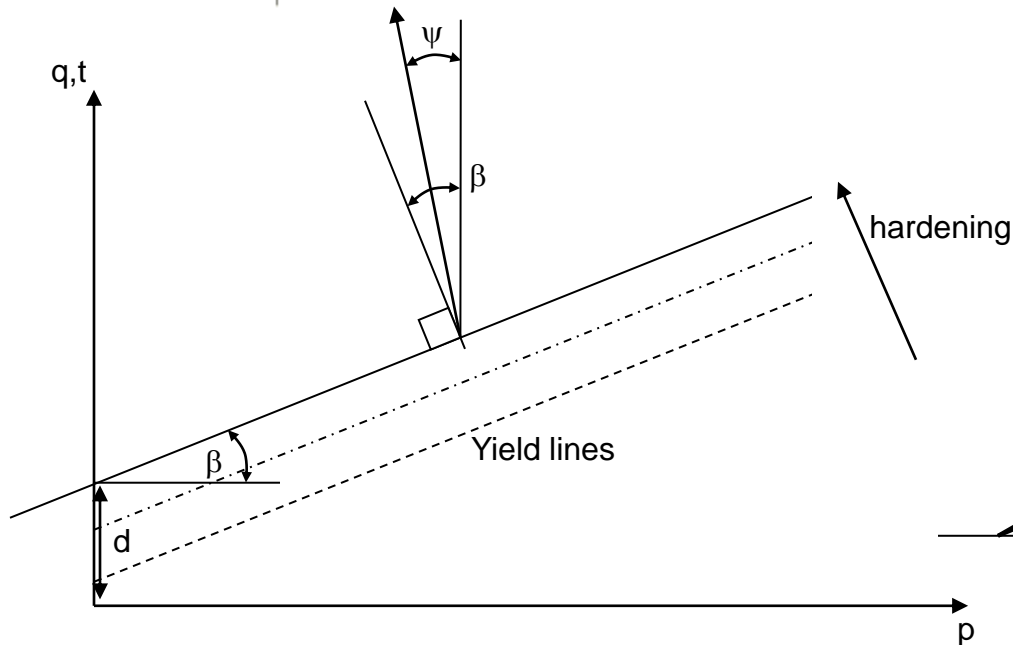
DP angle of friction = 55.6 degrees

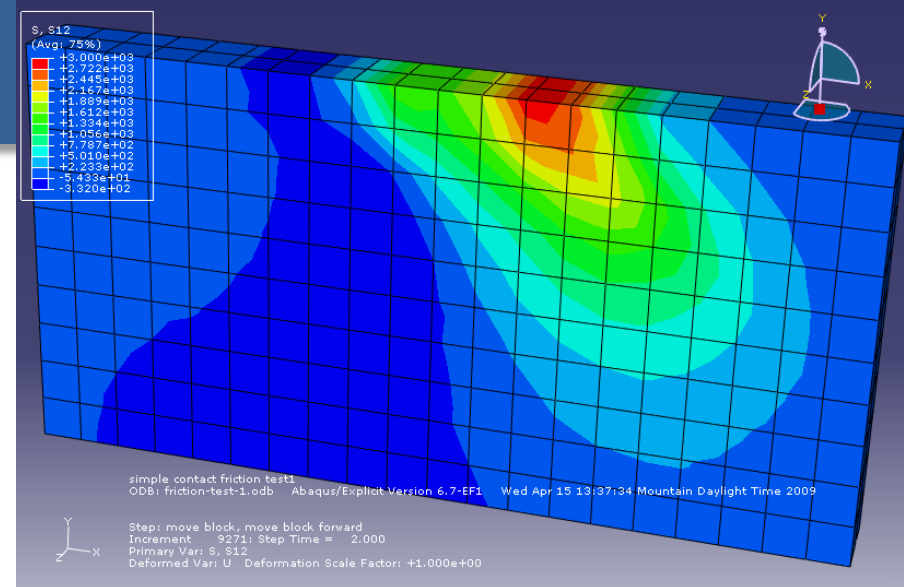
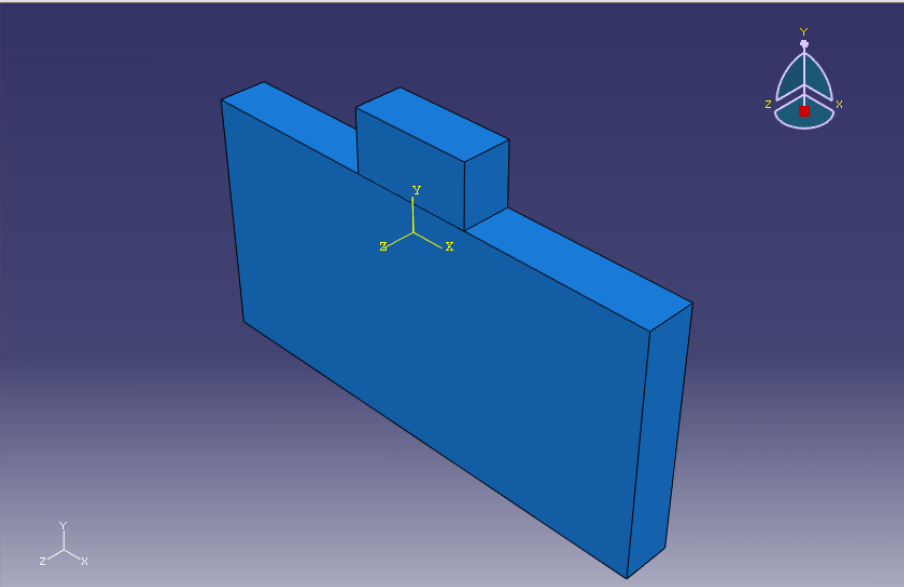
Flowstress ratio (K) = 0.79

Dilation Angle = 15 degrees

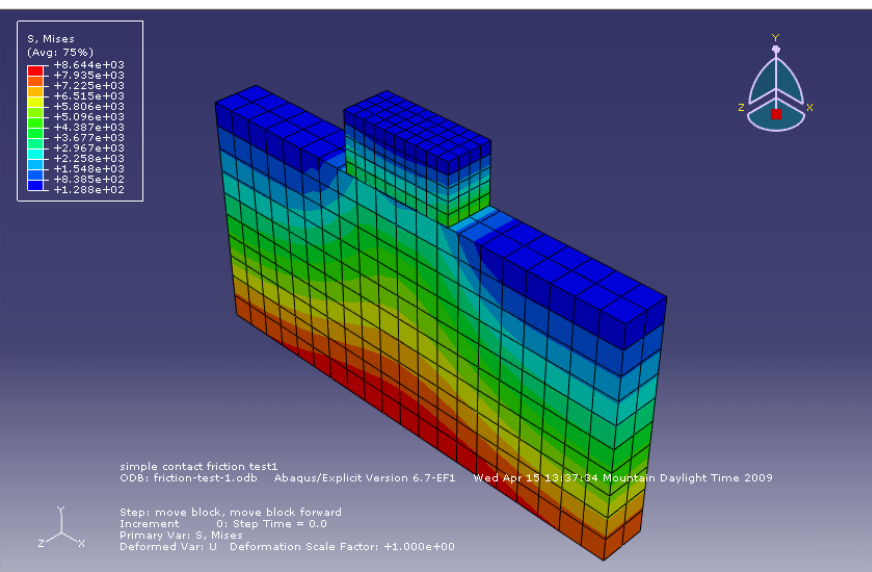
 $E = 36 \text{ Mpa}$
 $\nu = 0.3$

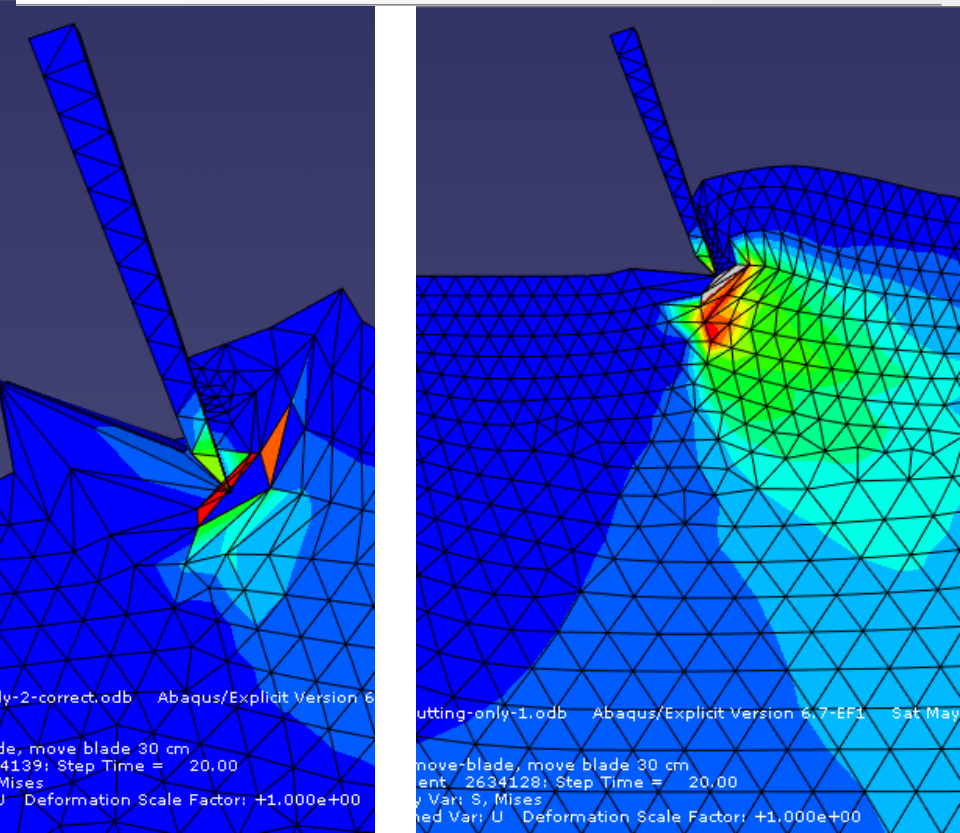
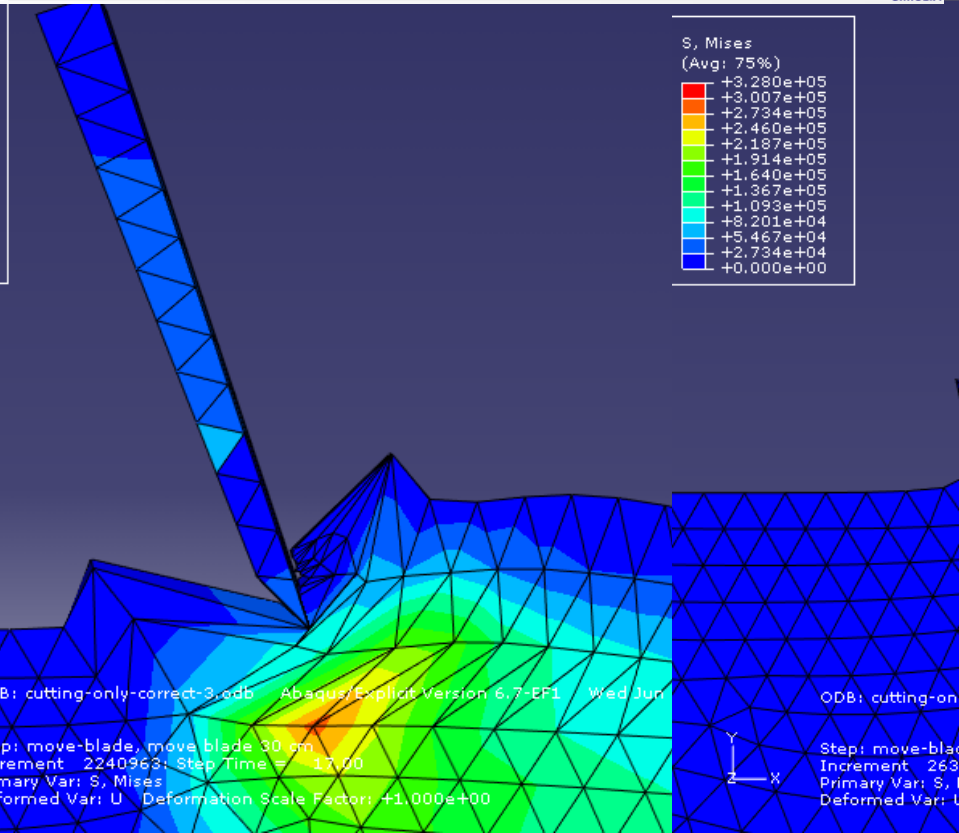
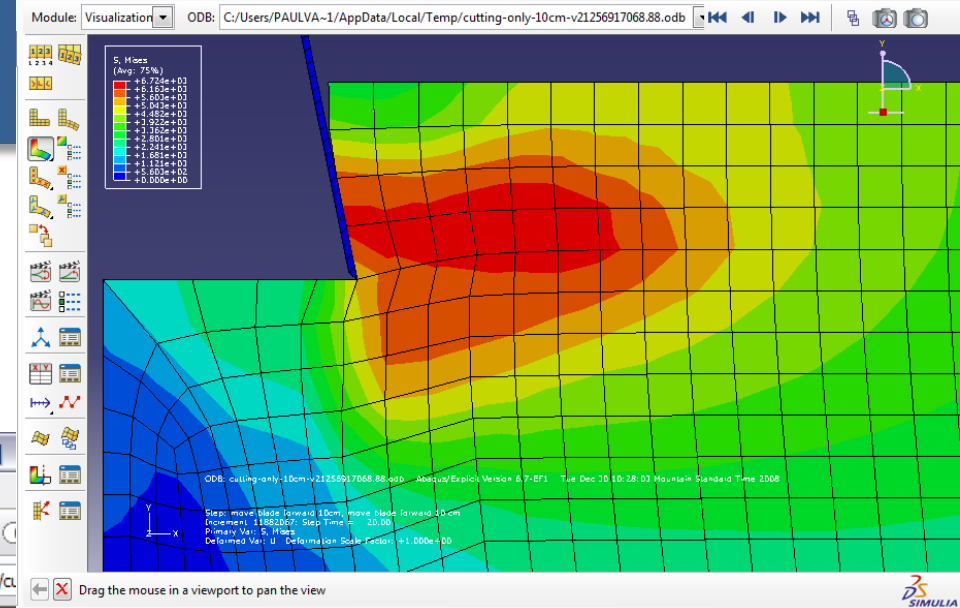
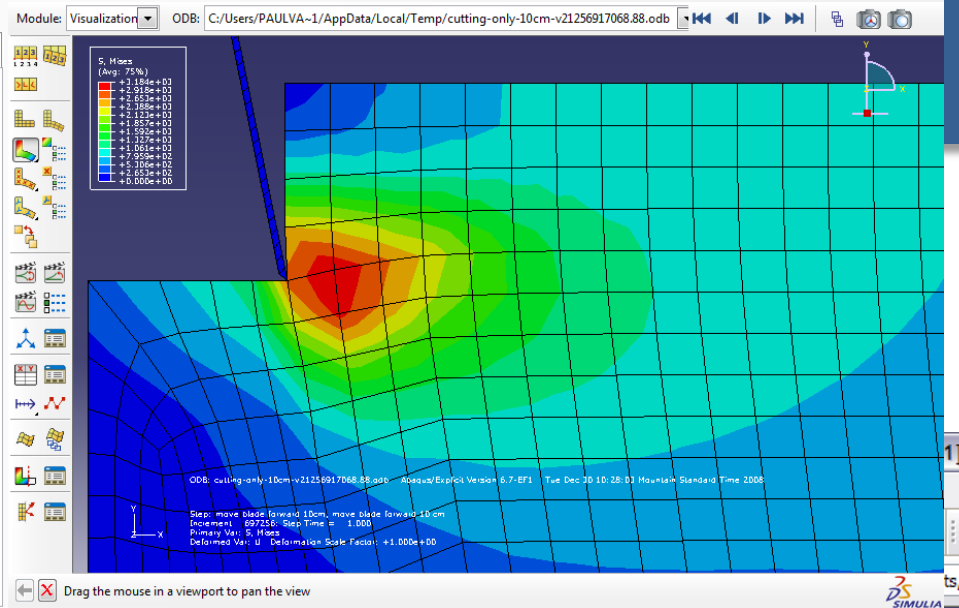
hardening 2	
yield stress (kPa)	absolute plastic strain
25	0
70	0.027
150	0.028

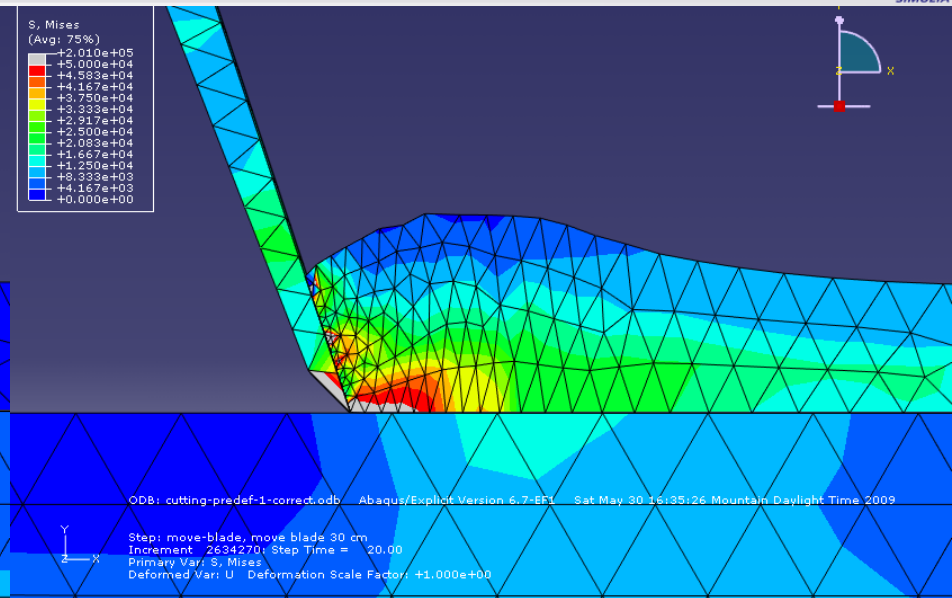
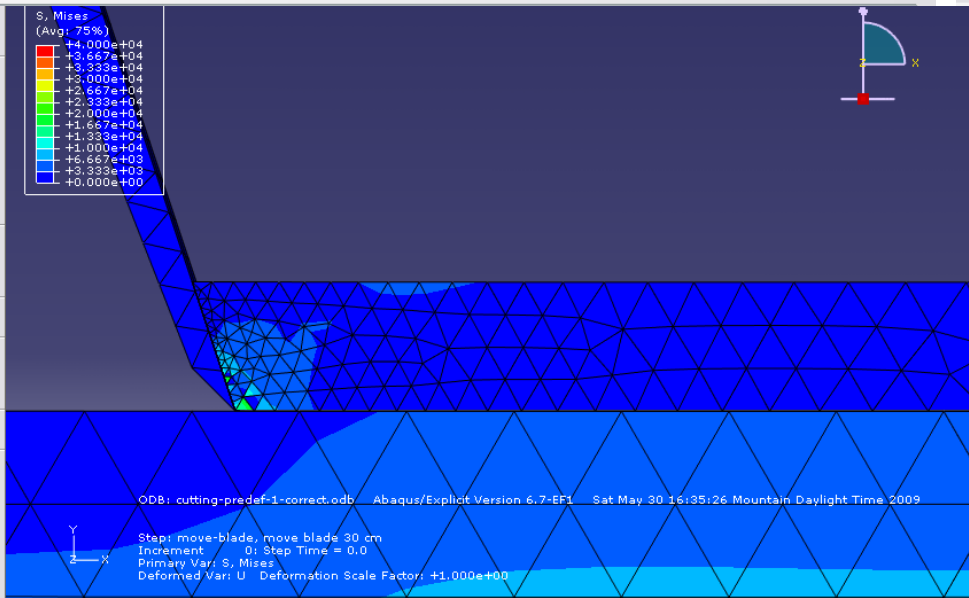
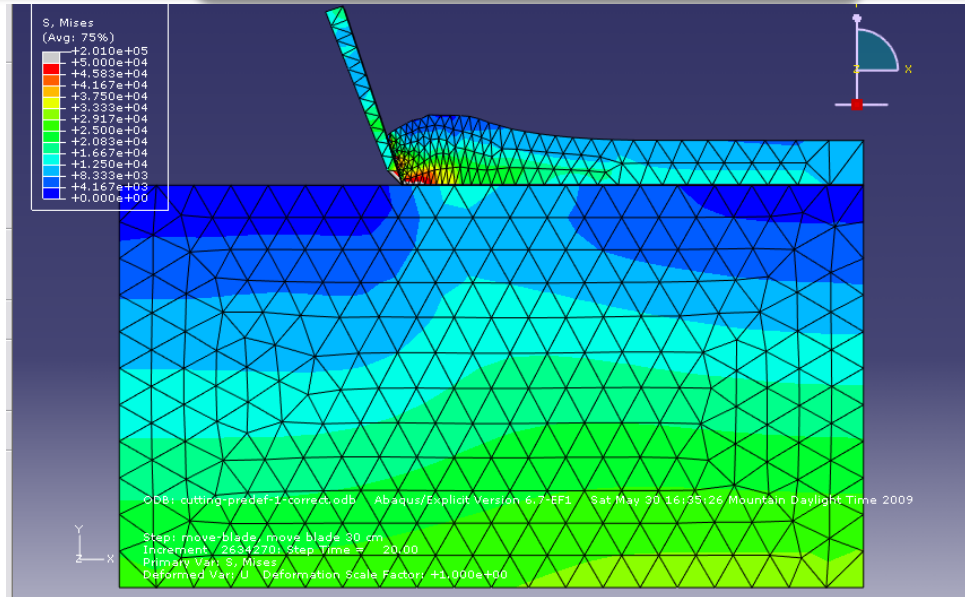
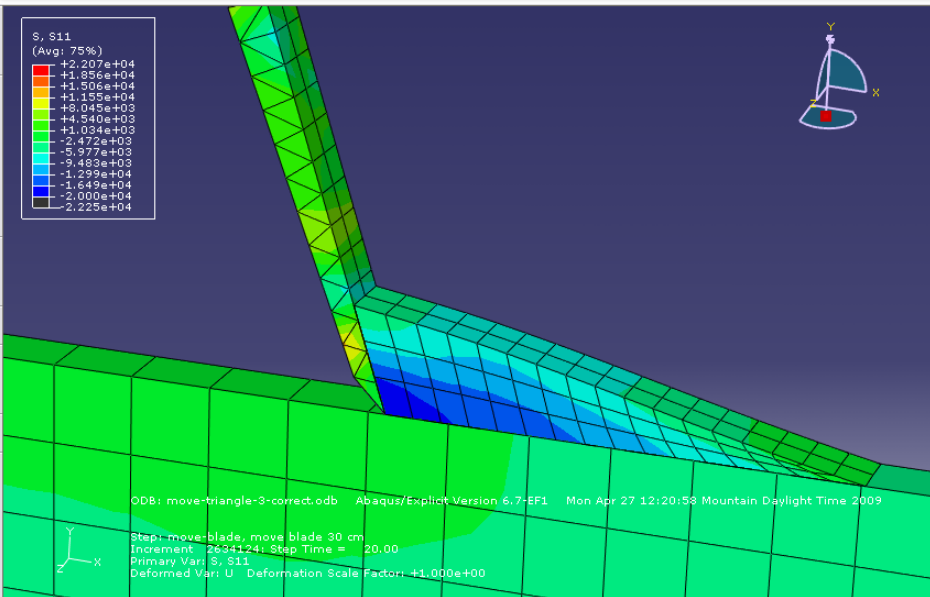


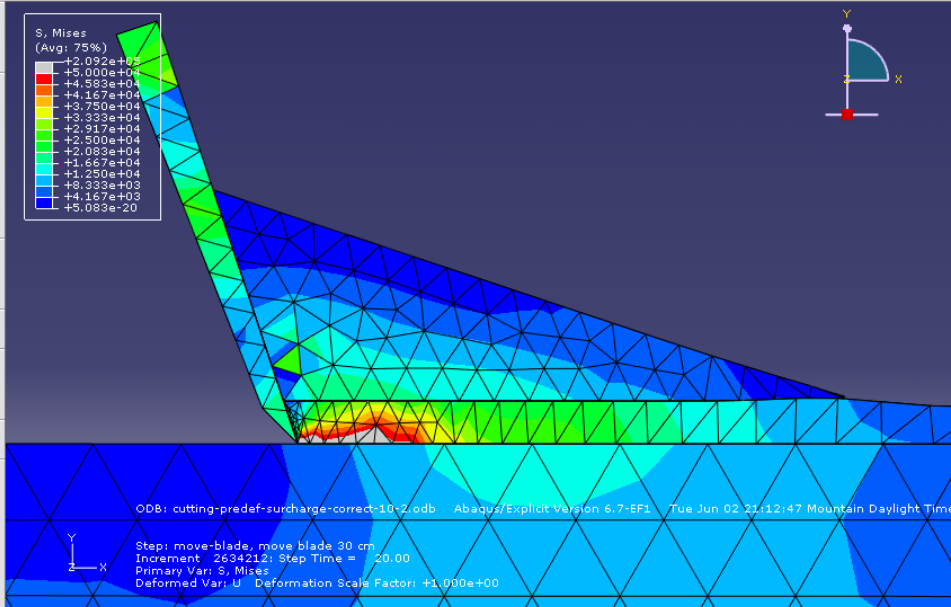


- Theoretical case
- Calibration of FEA model
- Damping in particular





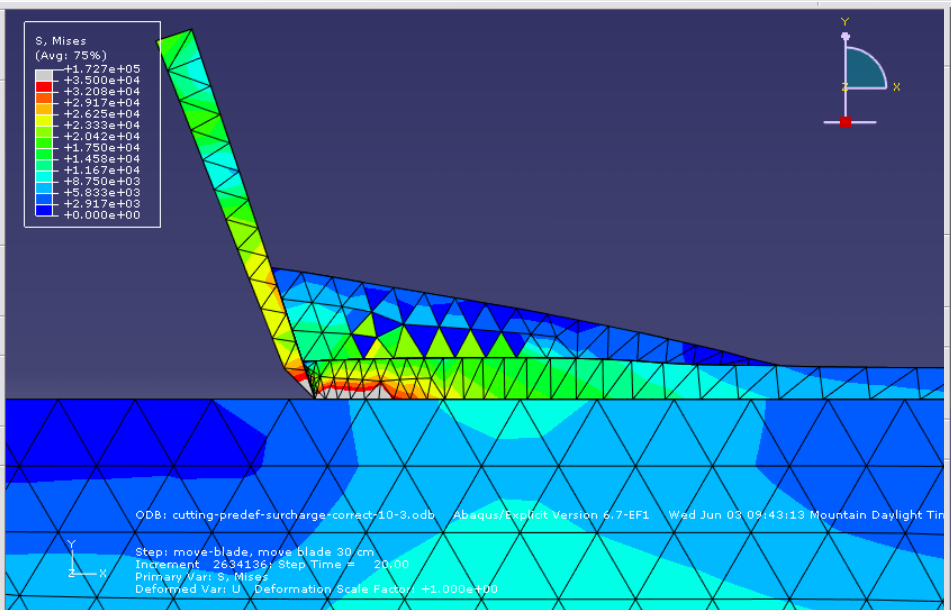




Drag the mouse in a viewport to define a box for zooming the view



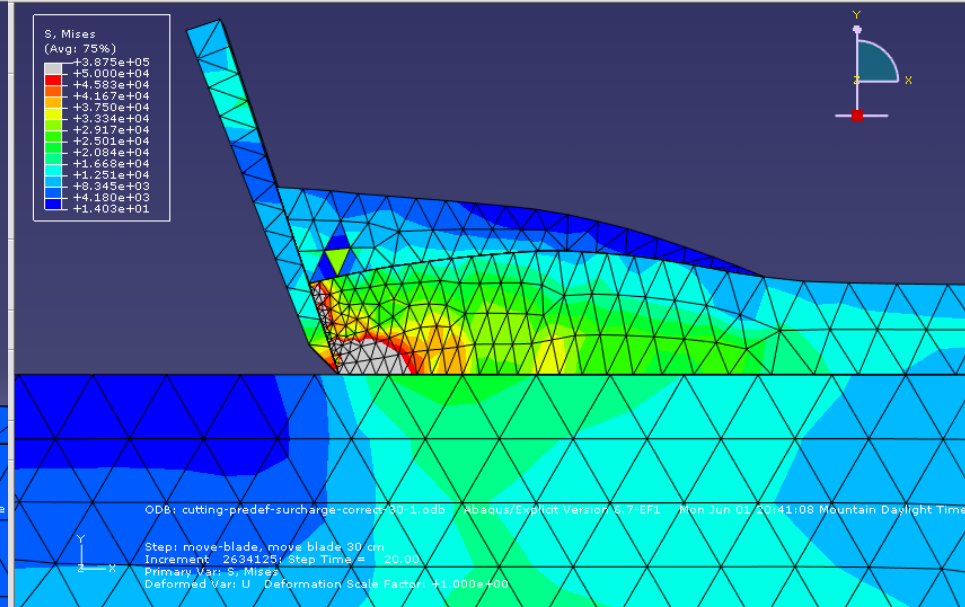
Drag the mouse in a viewport to define a box for zooming the view



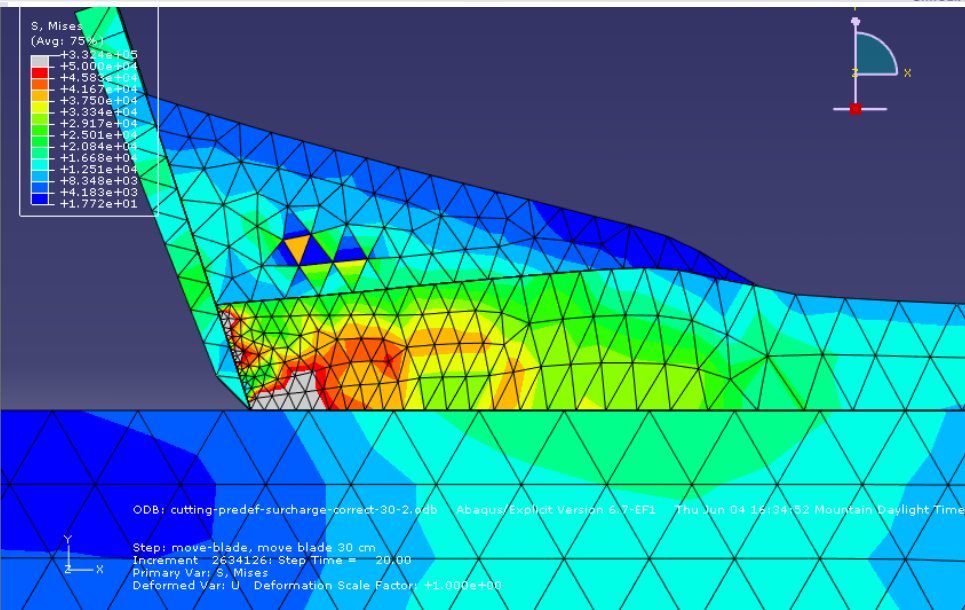
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Drag the mouse in a viewport to define a box for zooming the view

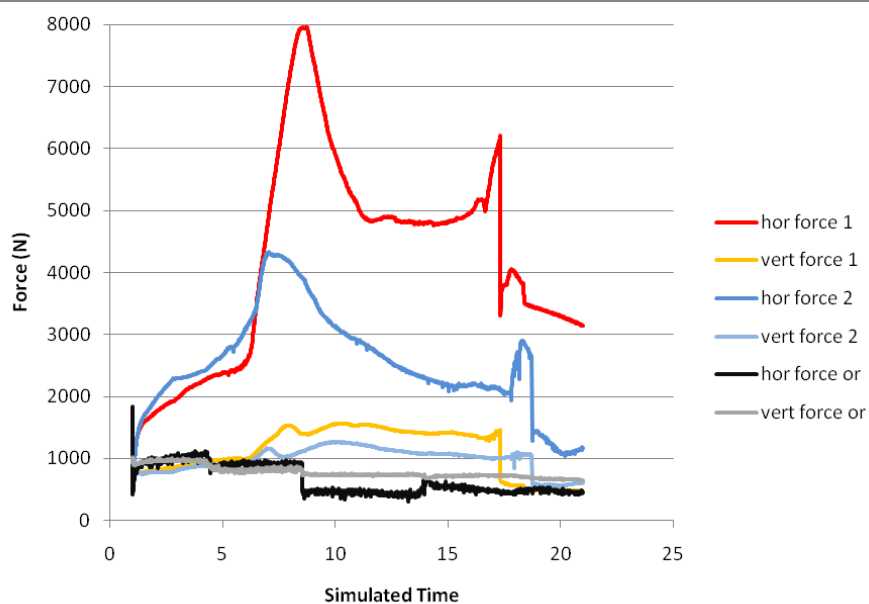
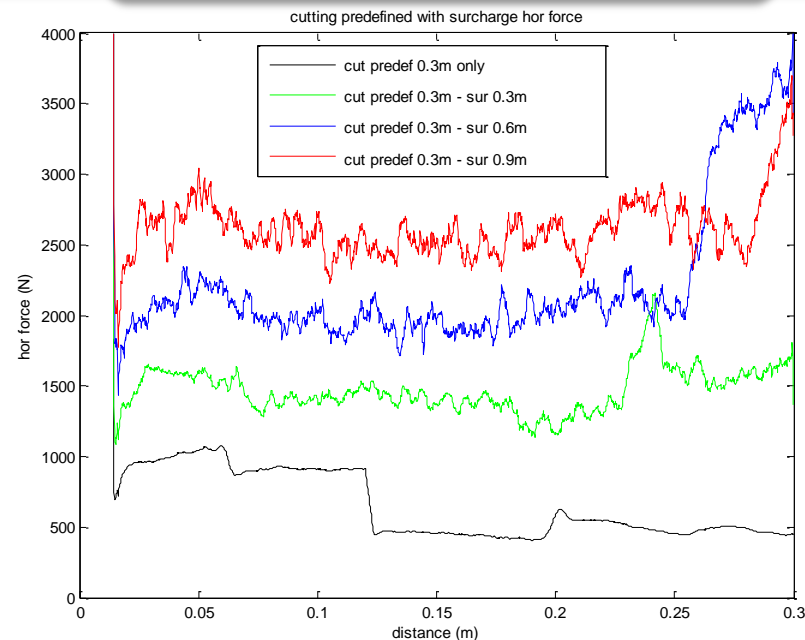
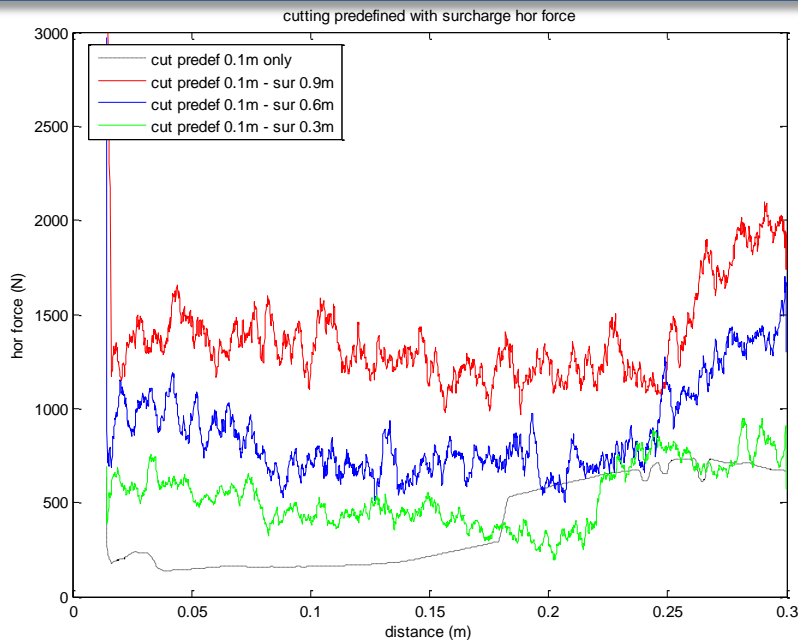


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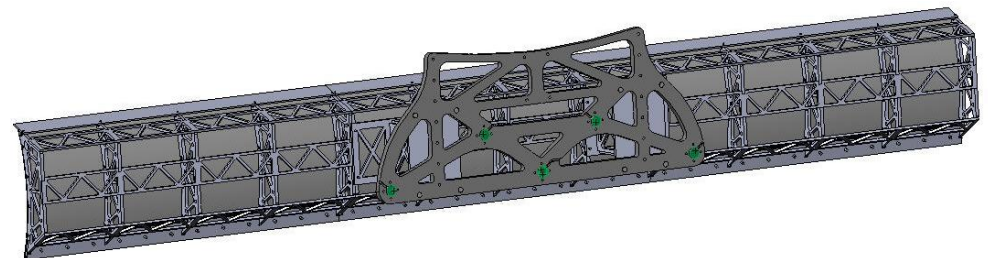
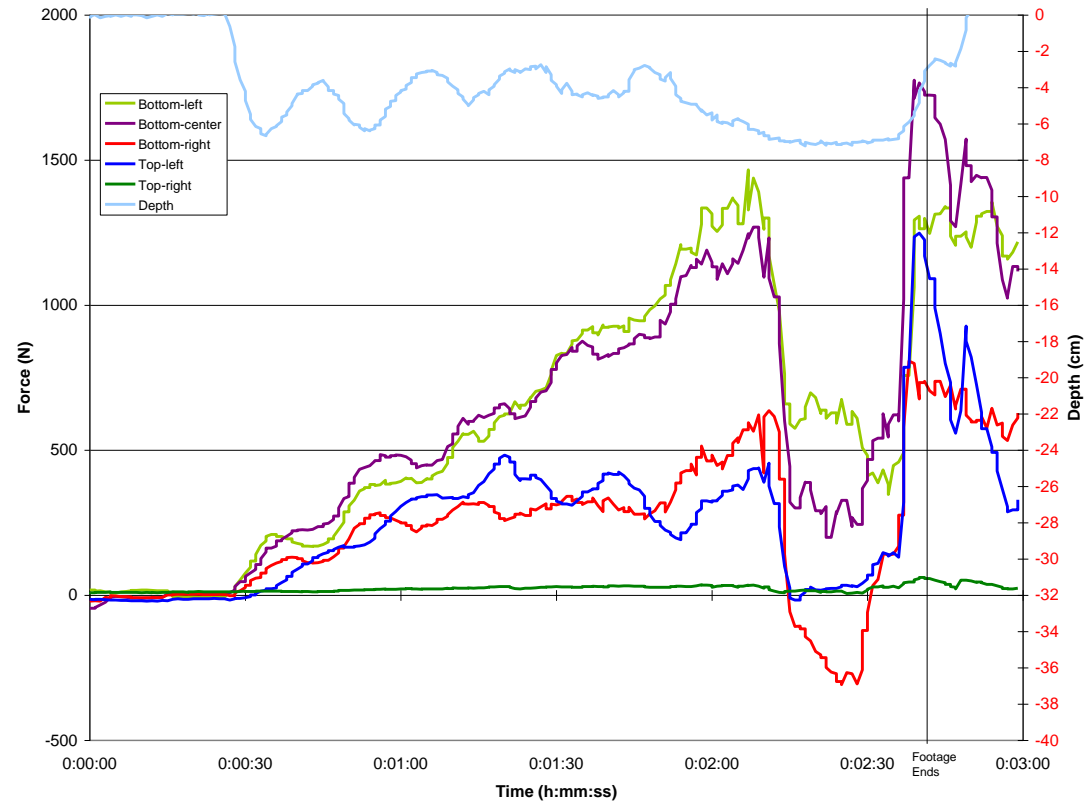
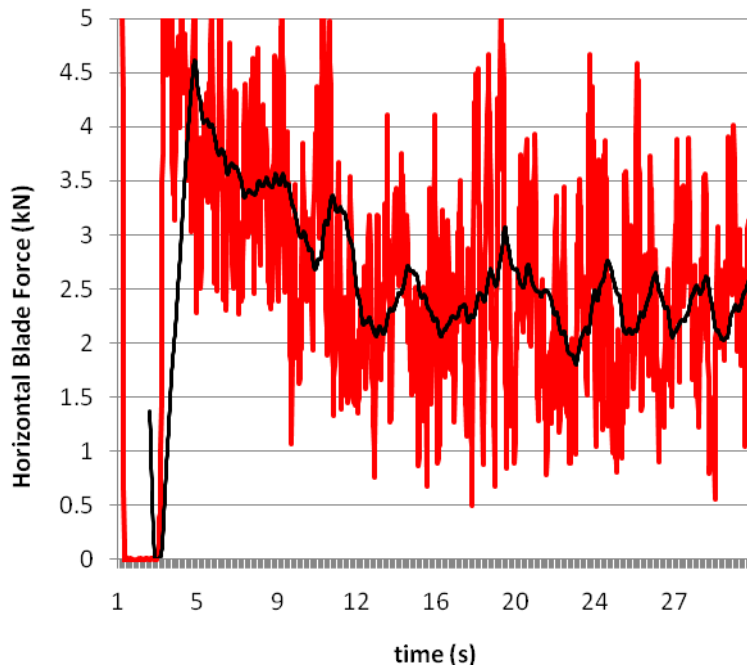
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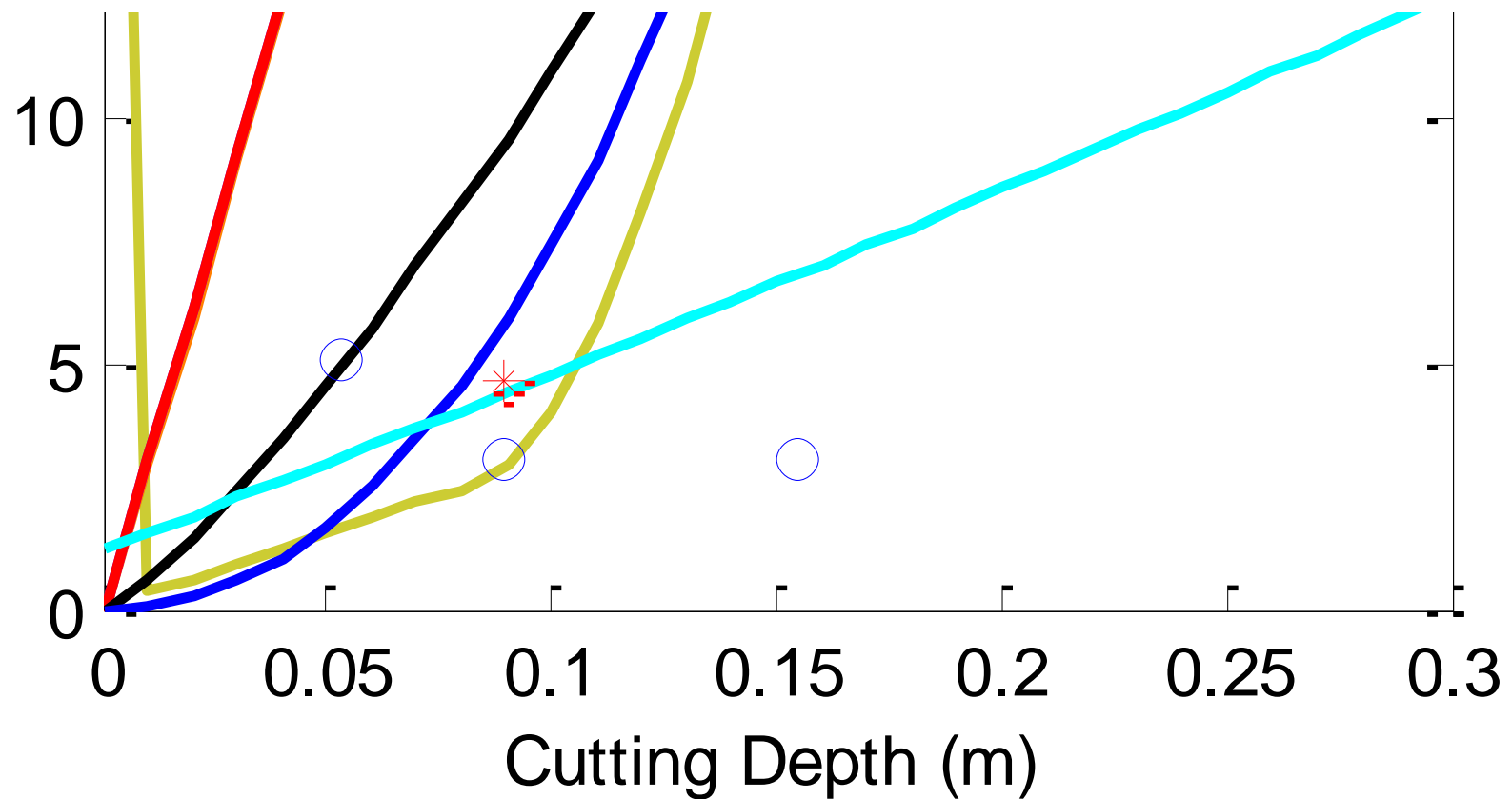




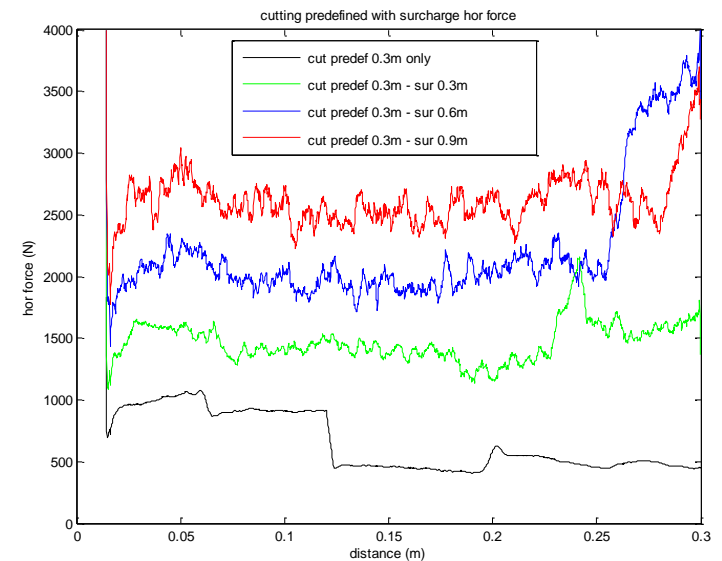
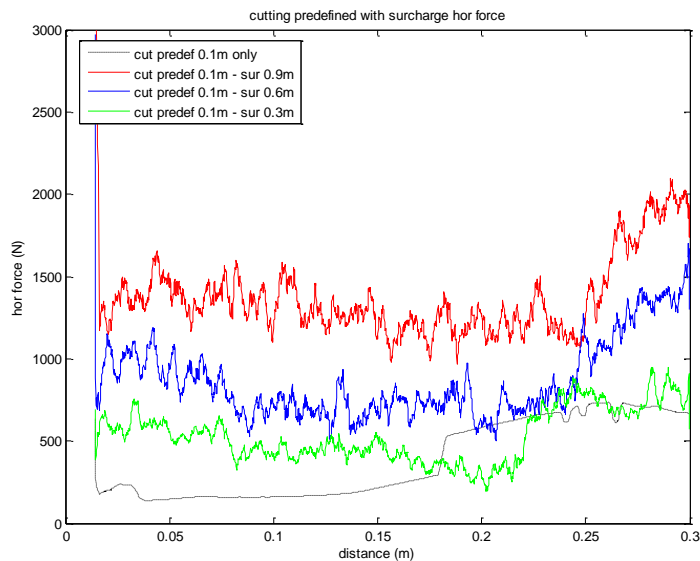
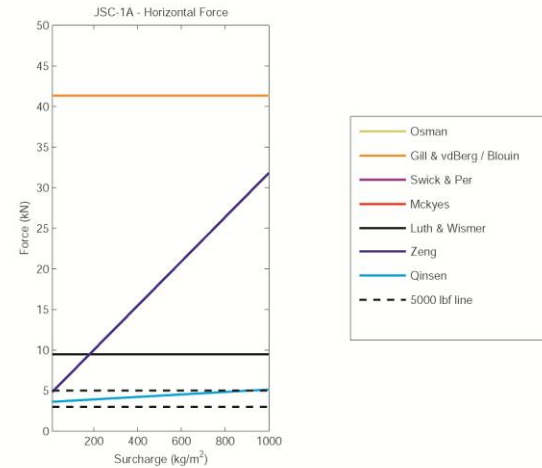
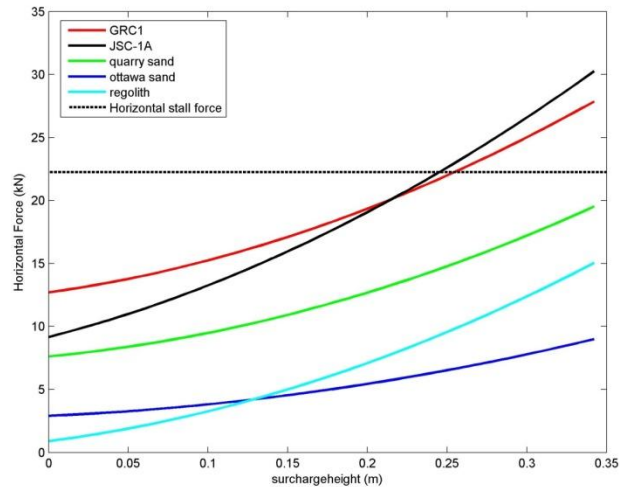
- Cut of 0.1 m & 0.3 m
- Surcharge 0.3, 0.6, 0.9 m
- Material model

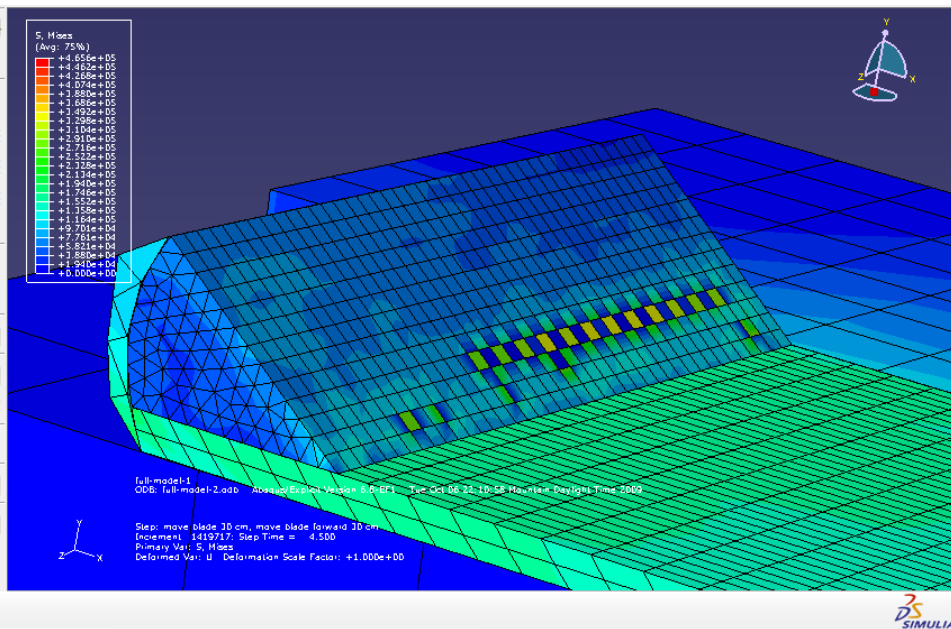
Experimental data and FEA results



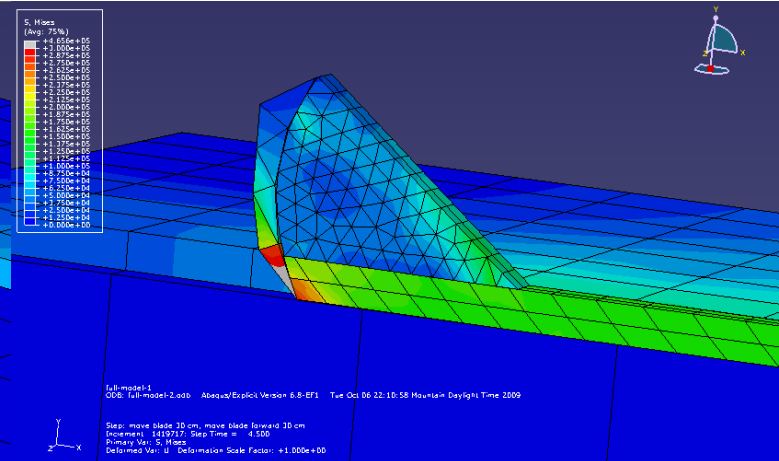
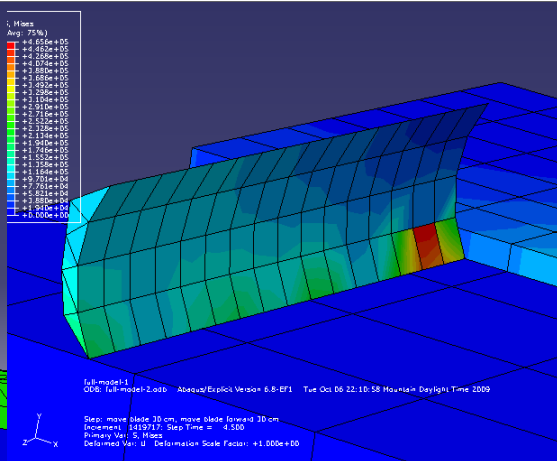
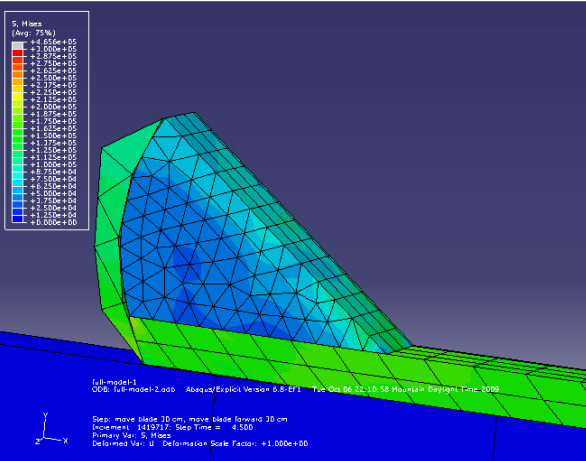


Effect of Surcharge





- Full size model
- Symmetric around center plane
- 3D FEA
- Constitutive model



Conclusions

- FEA modeling possible
- Gives good ballpark results
- Only can model first cut and first part of pushing
- Constitutive model problematic
- Large variability in soil properties
- Very little good experimental data available at the right scales and in the right materials
- FEA/DEM combo might yield more accurate time results
- Computationally intensive (full model takes 3-4 days to run one case)
- Future work to include FEA of small scale experiments completed in mean time

Thanks

- NASA Surface Systems (Rob Mueller)
- Dr. R. King
- Mark Gefreh, Andrew Brewer, Brian Dreiling

QUESTIONS?

